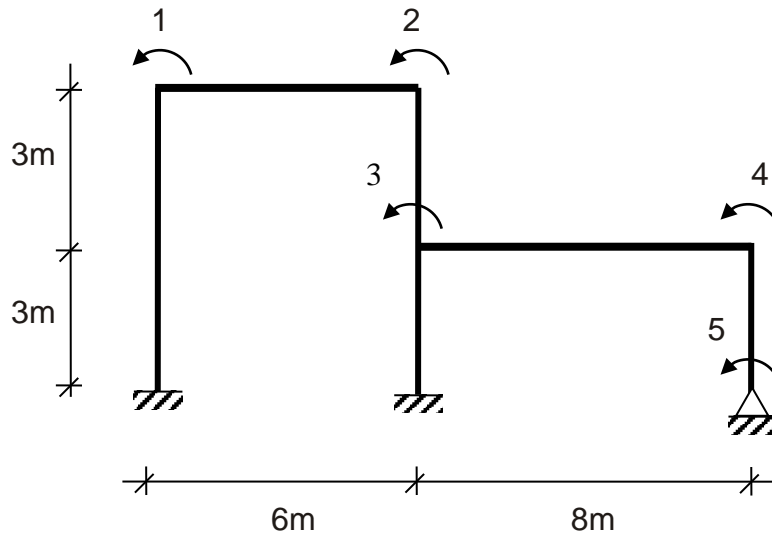


EXERCÍCIO DE MÉTODO DA RIGIDEZ – SOLUÇÃO GERAL

Um edifício foi modelado conforme desenho apresentado abaixo:



A partir da solução geral do método da rigidez pedem-se:

- A matriz de rigidez global da estrutura;
- O DMF para um carregamento uniformemente distribuído de 12kN/m aplicado sobre todas as vigas.

Dados:

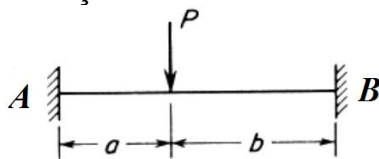


$$EJ_{VIGAS} = 4EJ_{PILARES}$$

$$[k_e] = \frac{2EJ}{L} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix}$$

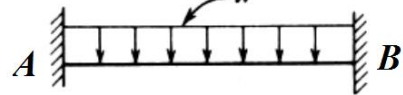
FORMULÁRIO

1) Reações de fixação:



$$M_A = \frac{Pab^2}{L^2} \quad M_B = -\frac{Pa^2b}{L^2}$$

$$R_A = \frac{Pb^2}{L^3} (3a + b) \quad R_B = \frac{Pa^2}{L^3} (a + 3b)$$



$$M_A = -M_B = \frac{wL^2}{12}$$

$$R_A = R_B = \frac{wL}{2}$$

2) Formulação do método da rigidez:

Solução Geral:

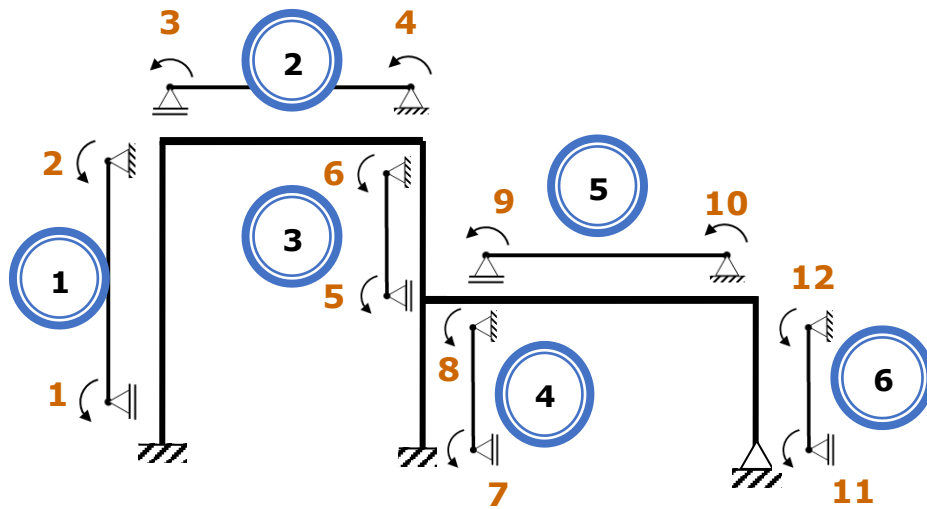
$$[K] = [A]^T \cdot [K_\ell] \cdot [A]$$

$$\{F\} = [K] \cdot \{r\}$$

$$\{S\} = \{S_0\} + [K_\ell] \cdot [A] \cdot \{r\}$$

Solução:

1) Desmembramento do pórtico em elementos e coordenadas locais:



2) Determinação da matriz de compatibilidade cinemática:

Coor global 1 = Coor locais 2 e 3

$$[A]_{2,1} = [A]_{3,1} = 1$$

Coor global 2 = Coor locais 4 e 6

$$[A]_{4,2} = [A]_{6,2} = 1$$

Coor global 3 = Coor locais 5, 8 e 9

$$[A]_{5,3} = [A]_{8,3} = [A]_{9,3} = 1$$

Coor global 4 = Coor locais 10 e 12

$$[A]_{10,4} = [A]_{12,4} = 1$$

Coor global 5 = Coor locais 11

$$[A]_{11,5} = 1$$

$$\Rightarrow [A] = \begin{bmatrix} 0 & 0 & 0 & 0 & 0 \\ 1 & 0 & 0 & 0 & 0 \\ 1 & 0 & 0 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 \\ 0 & 1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 1 & 0 & 0 \\ 0 & 0 & 0 & 1 & 0 \\ 0 & 0 & 0 & 0 & 1 \\ 0 & 0 & 0 & 1 & 0 \end{bmatrix}$$

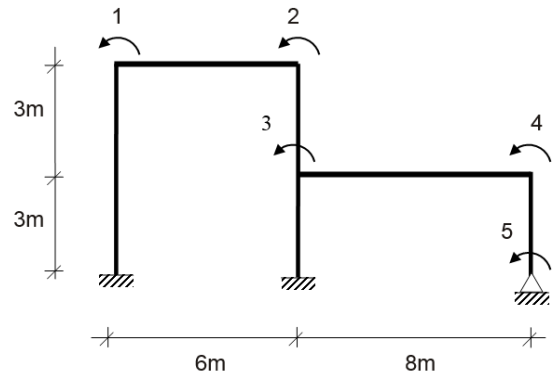
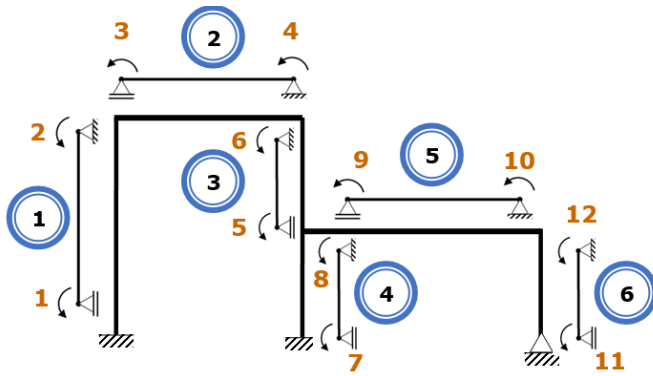
Command Window

```
>> A(2,1)=1;A(3,1)=1;
A(4,2)=1;A(6,2)=1;
A(5,3)=1;A(8,3)=1;A(9,3)=1;
A(10,4)=1;A(12,4)=1;A(11,5)=1;
>> A'
```

ans =

```
0 1 1 0 0 0 0 0 0 0 0 0
0 0 0 1 0 1 0 0 0 0 0 0
0 0 0 0 1 0 0 1 1 0 0 0
0 0 0 0 0 0 0 0 0 1 0 1
0 0 0 0 0 0 0 0 0 0 1 0
```

3) Determinação das matrizes de rigidez dos elementos:



$$[k_e] = \frac{2EJ}{L} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix}$$

Fazendo: $EJ_{VIGAS} = 4EJ_{PILARES} = 4EJ$

Elmto 1 ($L = 6$ m, rigidez EJ):

$$\Rightarrow [k_{e1}] = \frac{2EJ}{6} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix} = \frac{EJ}{3} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix}$$

Elmto 2 ($L = 6$ m, rigidez $4EJ$):

$$\Rightarrow [k_{e2}] = \frac{8EJ}{6} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix} = \frac{4EJ}{3} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix}$$

Elmtos 3, 4 e 6 ($L = 3$ m, rigidez EJ):

$$\Rightarrow [k_{e3}] = [k_{e4}] = [k_{e6}] = \frac{2EJ}{3} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix}$$

Elmto 5 ($L = 8$ m, rigidez $4EJ$):

$$\Rightarrow [k_{e5}] = \frac{8EJ}{8} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix} = EJ \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix}$$

Command Window

```
>> E=sym('E'); J=sym('J');
>>
```

Command Window

```
>> k1=E*J/3*[2 1;1 2]

k1 =

[ (2*E*J)/3, (E*J)/3]
[ (E*J)/3, (2*E*J)/3]

>> k2=4*E*J/3*[2 1;1 2];
>> k3=2*E*J/3*[2 1;1 2];
>> k4=k3;
>> k5=E*J*[2 1;1 2];
>> k6=k3;
fx >>
```

4) Determinação da matriz de rigidez global:

$$[K_\ell] = \begin{bmatrix} [k_{e1}] & & & & \\ & [k_{e2}] & & & \\ & & [k_{e3}] & & \\ & & & [k_{e4}] & \\ & & & & [k_{e5}] \end{bmatrix}$$

$$[K] = [A]^t \cdot [K_\ell] \cdot [A]$$

```
Command Window
>> K1=blkdiag(k1,k2,k3,k4,k5,k6)

K1 =

[ (2*E*J)/3, (E*J)/3, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0]
[ (E*J)/3, (2*E*J)/3, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0]
[ 0, 0, (8*E*J)/3, (4*E*J)/3, 0, 0, 0, 0, 0, 0, 0, 0]
[ 0, 0, (4*E*J)/3, (8*E*J)/3, 0, 0, 0, 0, 0, 0, 0, 0]
[ 0, 0, 0, 0, (4*E*J)/3, (2*E*J)/3, 0, 0, 0, 0, 0, 0]
[ 0, 0, 0, 0, (2*E*J)/3, (4*E*J)/3, 0, 0, 0, 0, 0, 0]
[ 0, 0, 0, 0, 0, 0, (4*E*J)/3, (2*E*J)/3, 0, 0, 0, 0]
[ 0, 0, 0, 0, 0, 0, (2*E*J)/3, (4*E*J)/3, 0, 0, 0, 0]
[ 0, 0, 0, 0, 0, 0, 0, 0, 2*E*J, E*J, 0, 0]
[ 0, 0, 0, 0, 0, 0, 0, 0, E*J, 2*E*J, 0, 0]
[ 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, (4*E*J)/3, (2*E*J)/3]
[ 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, (2*E*J)/3, (4*E*J)/3]

fx >>
```

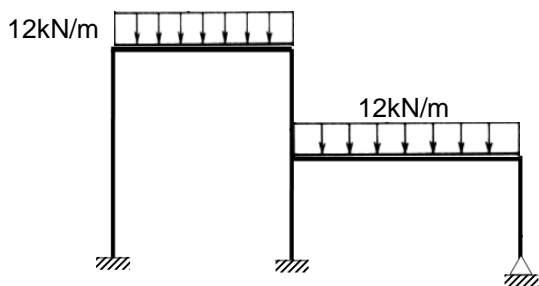
```
Command Window
>> K=A'*K1*A

K =

[ (10*E*J)/3, (4*E*J)/3, 0, 0, 0]
[ (4*E*J)/3, 4*E*J, (2*E*J)/3, 0, 0]
[ 0, (2*E*J)/3, (14*E*J)/3, E*J, 0]
[ 0, 0, E*J, (10*E*J)/3, (2*E*J)/3]
[ 0, 0, 0, (2*E*J)/3, (4*E*J)/3]
```

5) Carregamento:

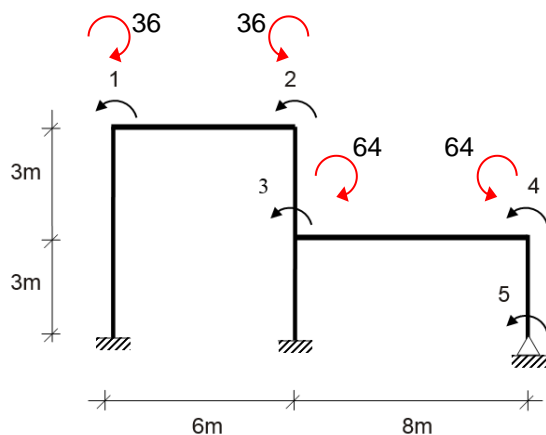
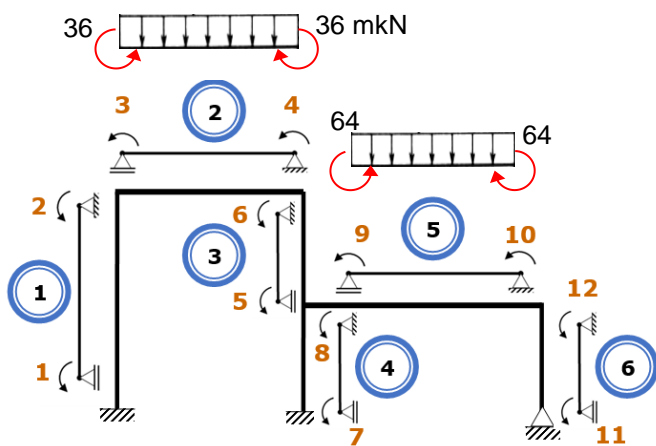
Carregamento distribuído:



Reações de Fixação

+

Carregamento Nodal Equivalente



$$\{S_0\} = \begin{Bmatrix} 0 \\ 0 \\ 36 \\ -36 \\ 0 \\ 0 \\ 0 \\ 0 \\ 64 \\ -64 \\ 0 \\ 0 \end{Bmatrix} m \cdot KN$$

$$\{F\} = \begin{Bmatrix} -36 \\ 36 \\ -64 \\ 64 \\ 0 \end{Bmatrix} m \cdot KN$$

```

Command Window
>> F=[-36 36 -64 64 0]';S0=[0 0 36 -36 0 0 0 0 64 -64 0 0]';
>> F'
ans =
    -36     36    -64     64     0
>> S0'
ans =
     0     0     36    -36     0     0     0     0     64    -64     0     0
    
```

6) Equilíbrio:

$$\{F\} = [K] \cdot \{r\}$$

```

Command Window
>> r=inv(K)*F

r =

-1505/(82*E*J)
3097/(164*E*J)
-1853/(82*E*J)
2367/(82*E*J)
-2367/(164*E*J)

```

7) Esforços:

$$\{S\} = \{S_0\} + [K_e] \cdot [A] \cdot \{r\}$$

```

Command Window
>> S=S0+K1*A*r;
>> S'*246

ans =

[ -1505, -3010, 3010, -2488, -4315, 2488, -3706, -7412, 11727, -7101, 0, 7101]

```

